

T+612 9319 2955 ABN: 48 942 921 969 Nominated Architects: Andrew Hipwell 662 Daniel Beekwilder 6192 63 Myrtle Street 63 Myrtle Street Chippendale NSW 2008 Sydney Australia djrd.com.au

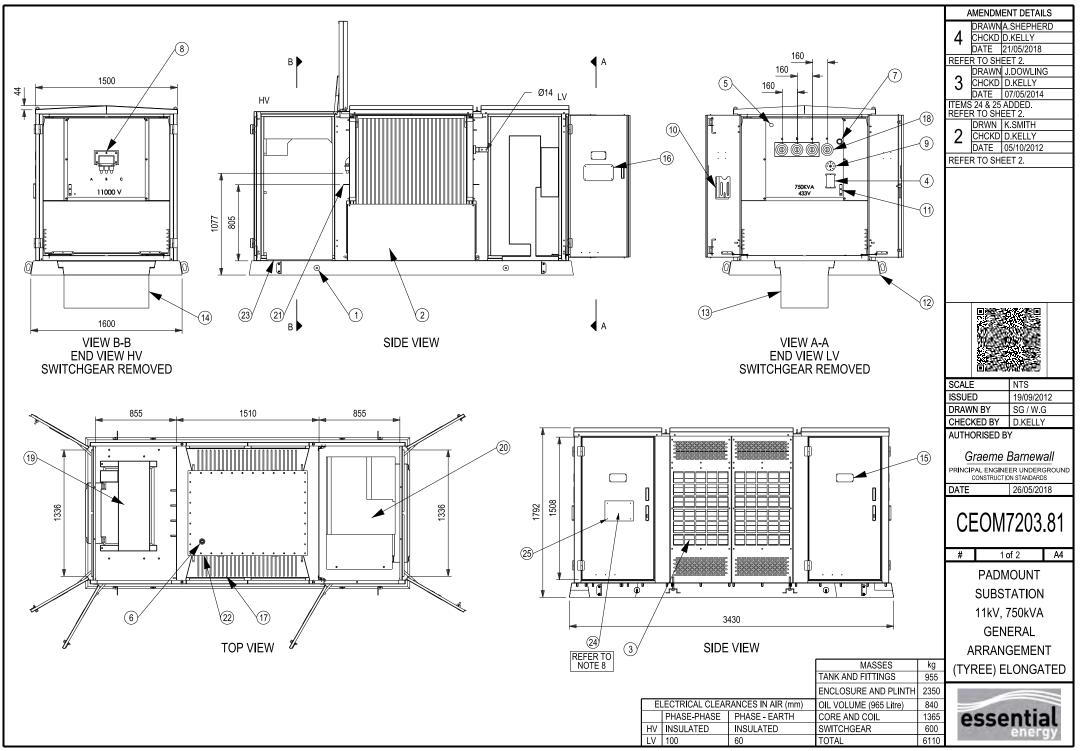
Project: Merritts Gondola, Thredbo NSW 2625 Client: Kosciuszko Thredbo Pty Ltd

Project No. 18 407 SK0.13 Drawing No.

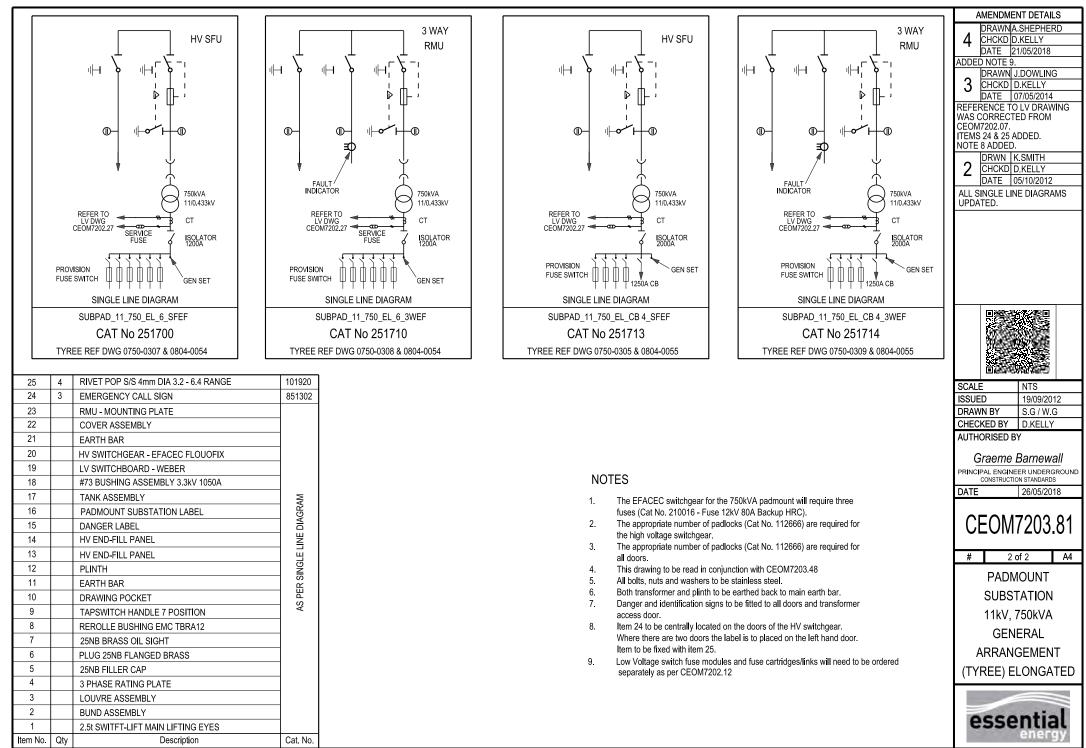
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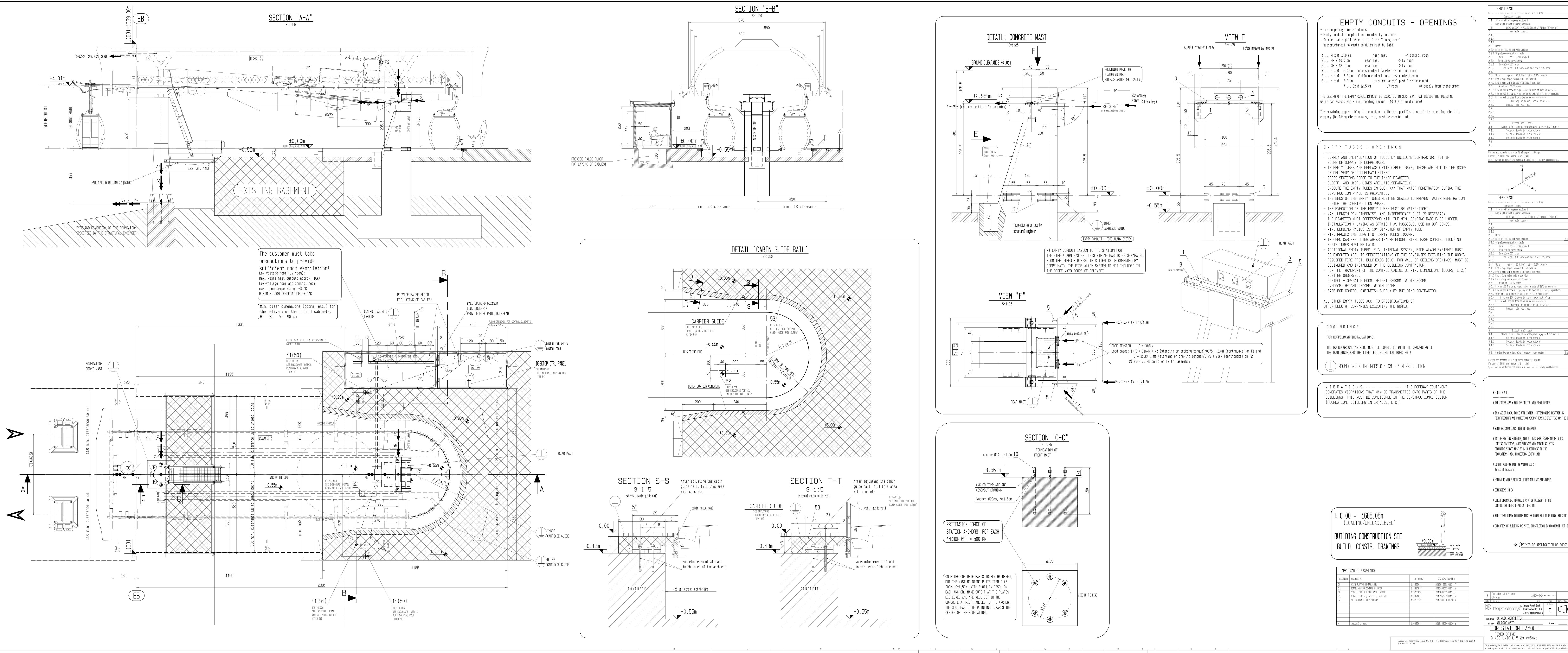
Revision Date: 19/08/19 Scale 1:400 @ A3 G

Revision:



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FRONT MAST	WAAOO	04672		UPHILL	03/05	5/2019
onnection forces on the connection point (acc.to drwg.)	8-MGD	MERRI	TTS			
Constant loads .1 Dead weight of ropeway equipment	Fx ± 10	Fy	Fz 205	Mx ± 70	My ± 50	Mz
.2 Dead weight of roof or compact enclosure	± 10		110	⊥ /v		
.3 DEAD WEIGHT - FIXED DRIVE / FIXED RETURN ST.			70		± 50	
2.1 Variable loads						
2.1.1						
2.1.2 2.2 Ropes						
2.2.1 Rope deflection and rope tension	-		60	± 20		
2.2.2 Signal/communication cable			30			
2.3 Snow (qs = 6.10 kN/m²) 2.3.1 Both sides 100% snow			425		± 40	
2.3.2 One side 50% snow			110	± 180	± 10	
2.3.3 One side 100% snow and one side 50% snow	_		320	± 180	± 30	
2.4 Wind (qa = 1.20 kN/m²; qi = 0.25 kN/m²)						
2.4.1 Wind at right angles to axis of lift in operation		± 15		± 90		
2.4.2 Wind at right angles to axis of lift out of operation 2.5 Wind on 100 % snow	_	± 55		± 425		
2.5.1 Wind on 100 % snow at right angles to axis of lift in operation		± 10		± 55		
2.5.2 Wind on 100 % snow at right angles to axis of lift out of operation		± 25		± 265		
P.6       Forces and torques from drive or return machinery         P.6.1       Starting or brake torque or 2.6.2		± 15		± 115		
2.6.2 Unequal tie rod load	-	± 10		± 125		
2.7						
2.7.1						
2.7.3						
Exceptional loads						
B.1Seismic influences (earthquake a_xy = 1.37 m/s²)B.1.1Seismic loads in x-direction	_		± 10			
3.1.2 Seismic loads in y-direction		± 55		± 410		
8.1.3 Seismic loads in z-direction 8.2			± 35	Ī	Ī	
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).2 ).3						
8.3						
8.3 orces and moments apply to final capacity design						
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B.3 Forces and moments apply to final capacity design Forces in [kN] and moments in [kNm].						
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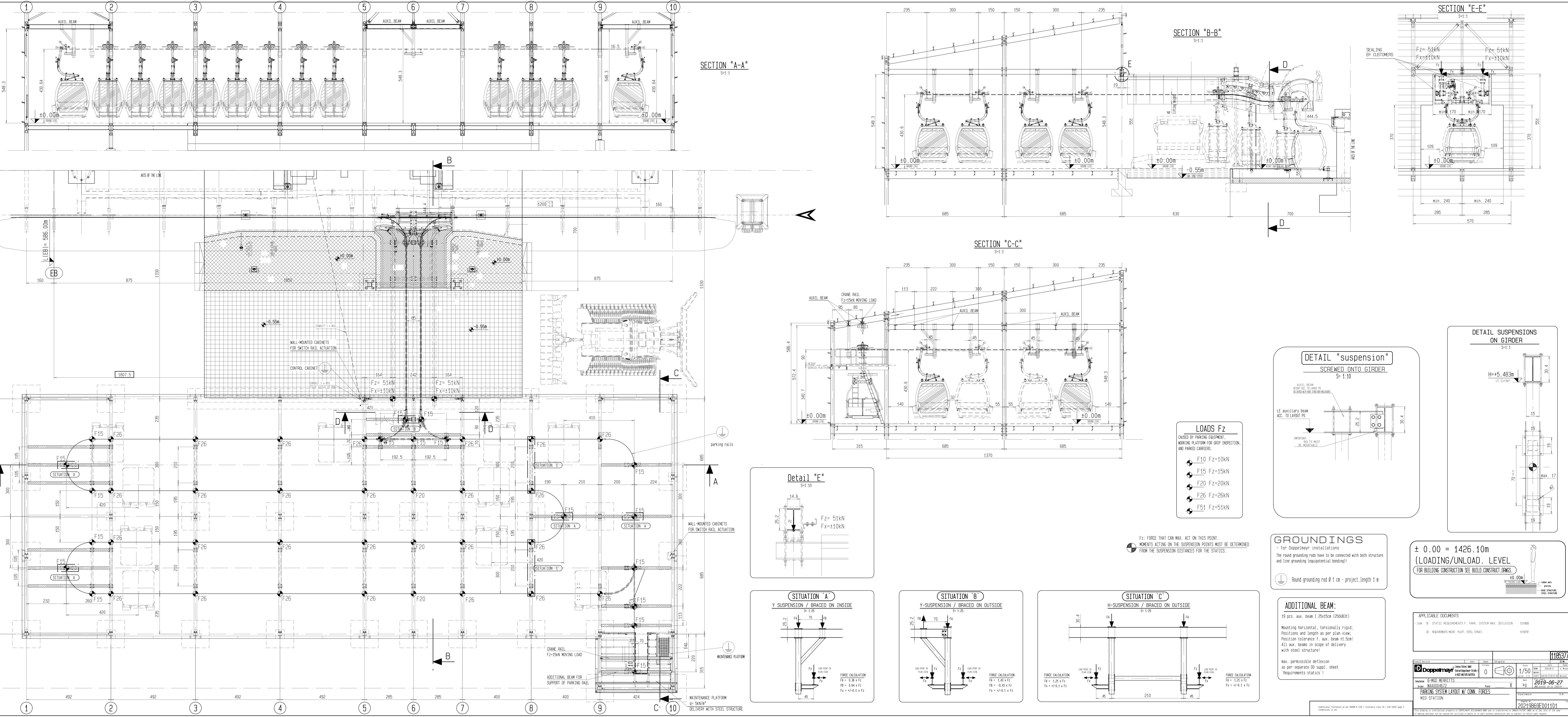
2 s = 719 kN see detail drawing concrete mast

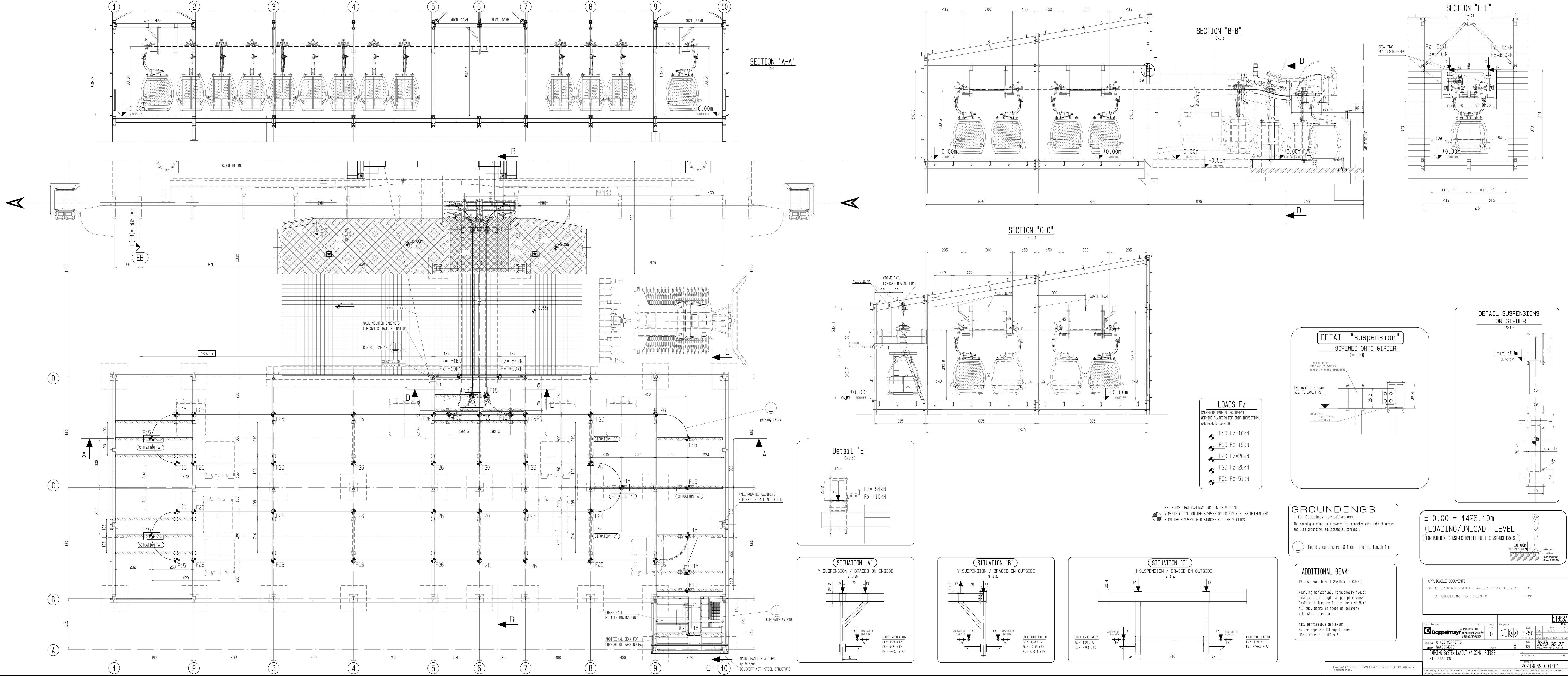
REINFORCEMENTS AND PROTECTION AGAINST TENSILE SPLITTING MUST BE CONSIDERED

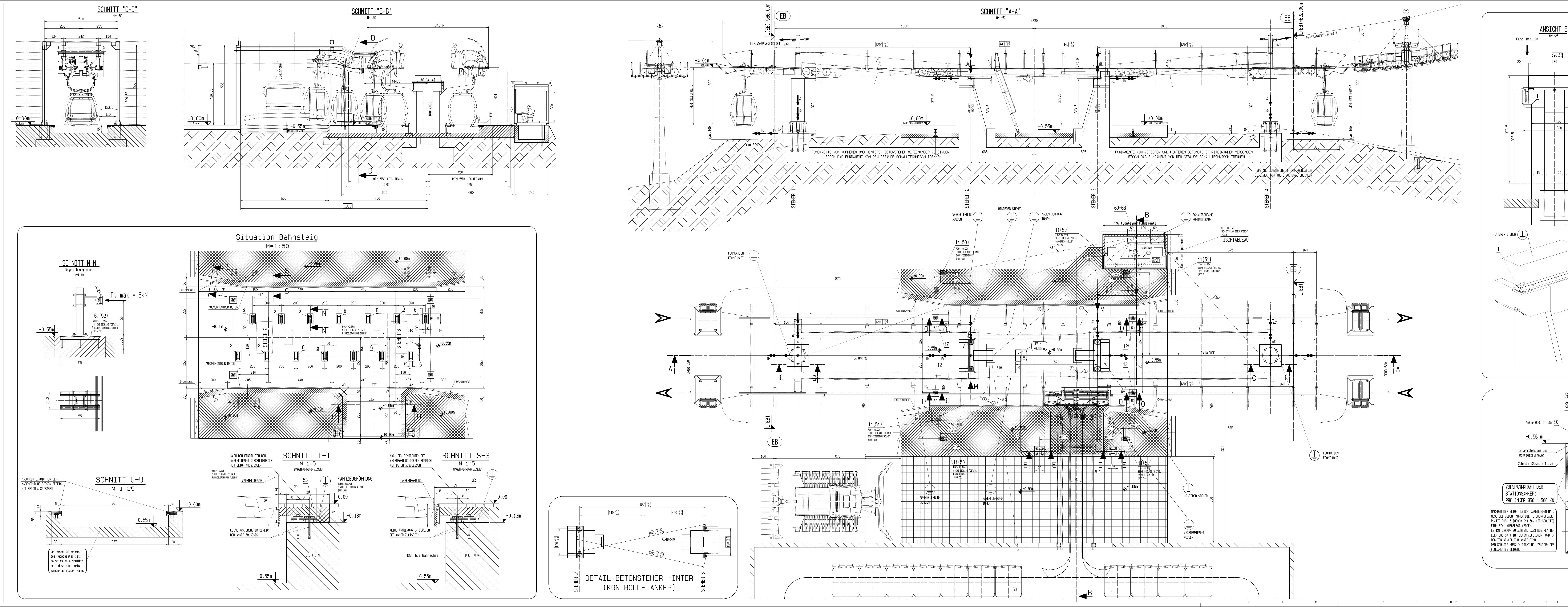
- \* TO THE STATION SUPPORTS, CONTROL CABINETS, CABIN GUIDE RAILS,
- \* ADDITIONAL EMPTY CONDUITS MUST BE PROVIDED FOR INTERNAL ELECTRIC SYSTEM!
- \* EXECUTION OF BUILDING AND STEEL CONSTRUCTION IN ACCORDANCE WITH DRAWINGS FOR BUIDLING/STEEL CONSTRUCTIONS!

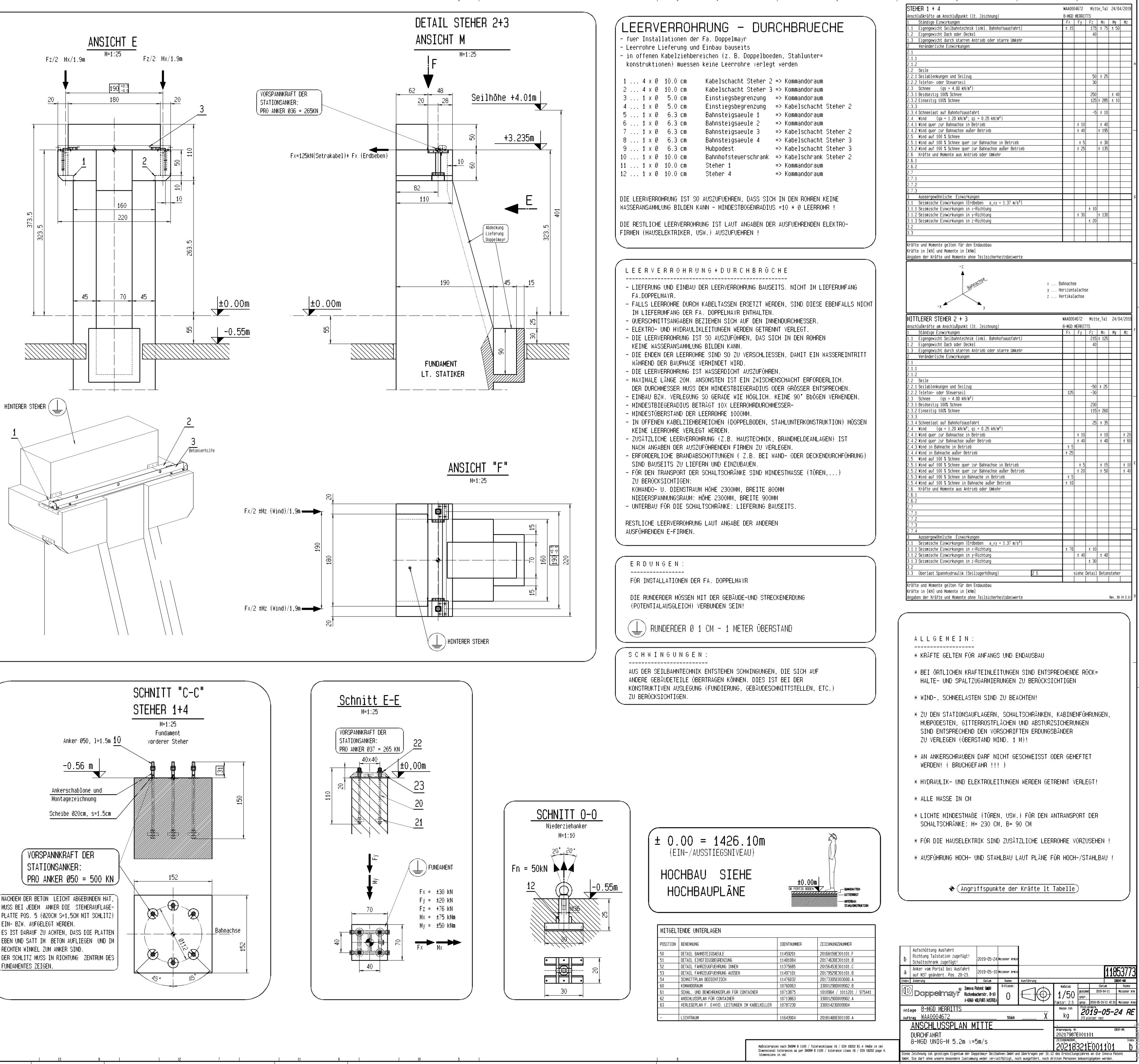
igoplus (POINTS OF APPLICATION OF FORCES ACC. TO TABLE)

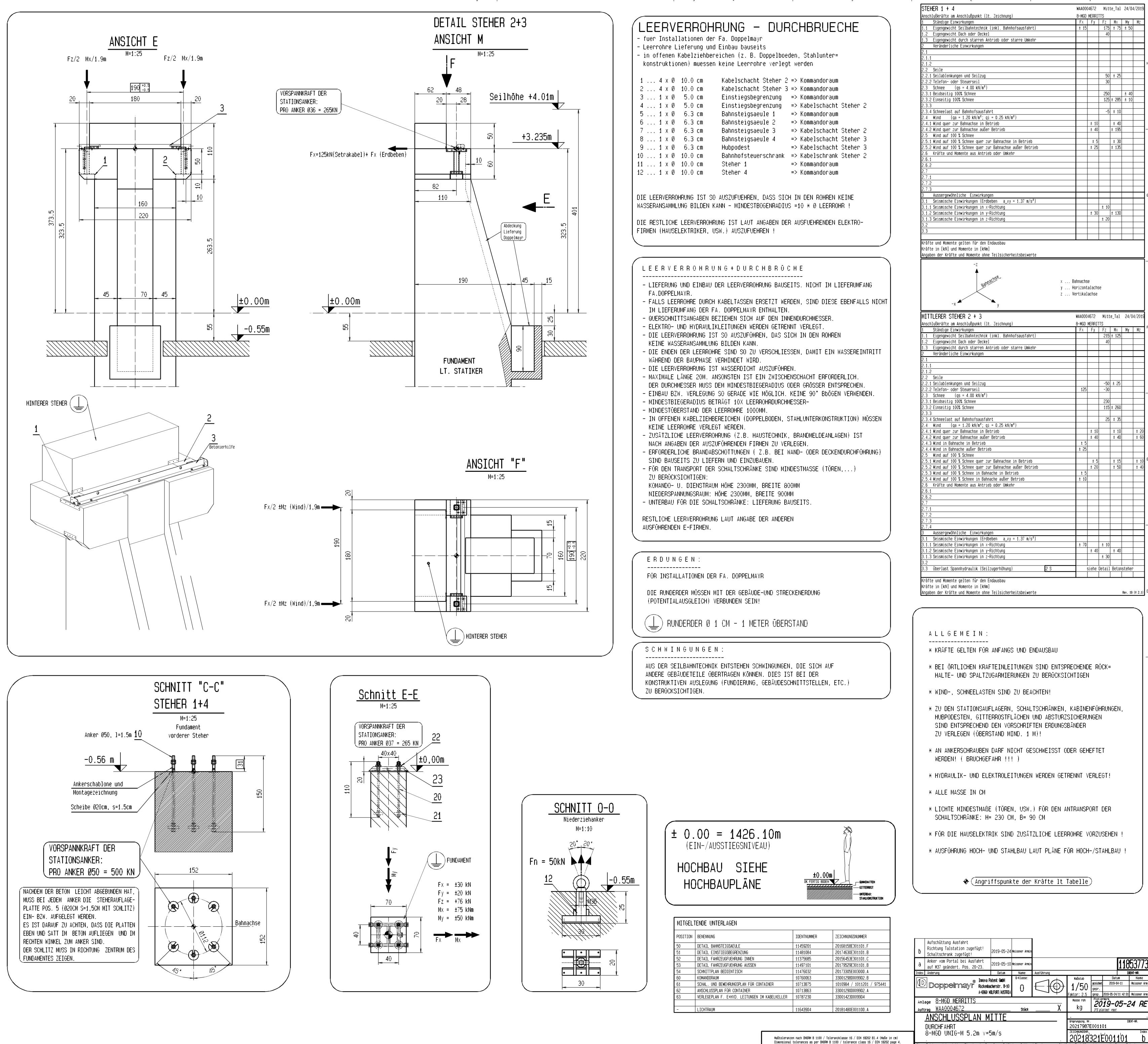
A	Position of LV room changed.	2019-05-14	Meissner Armin				118	53786
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	FIXED DRIVE				Original drawing no. 20217987		01.a	ID NO.
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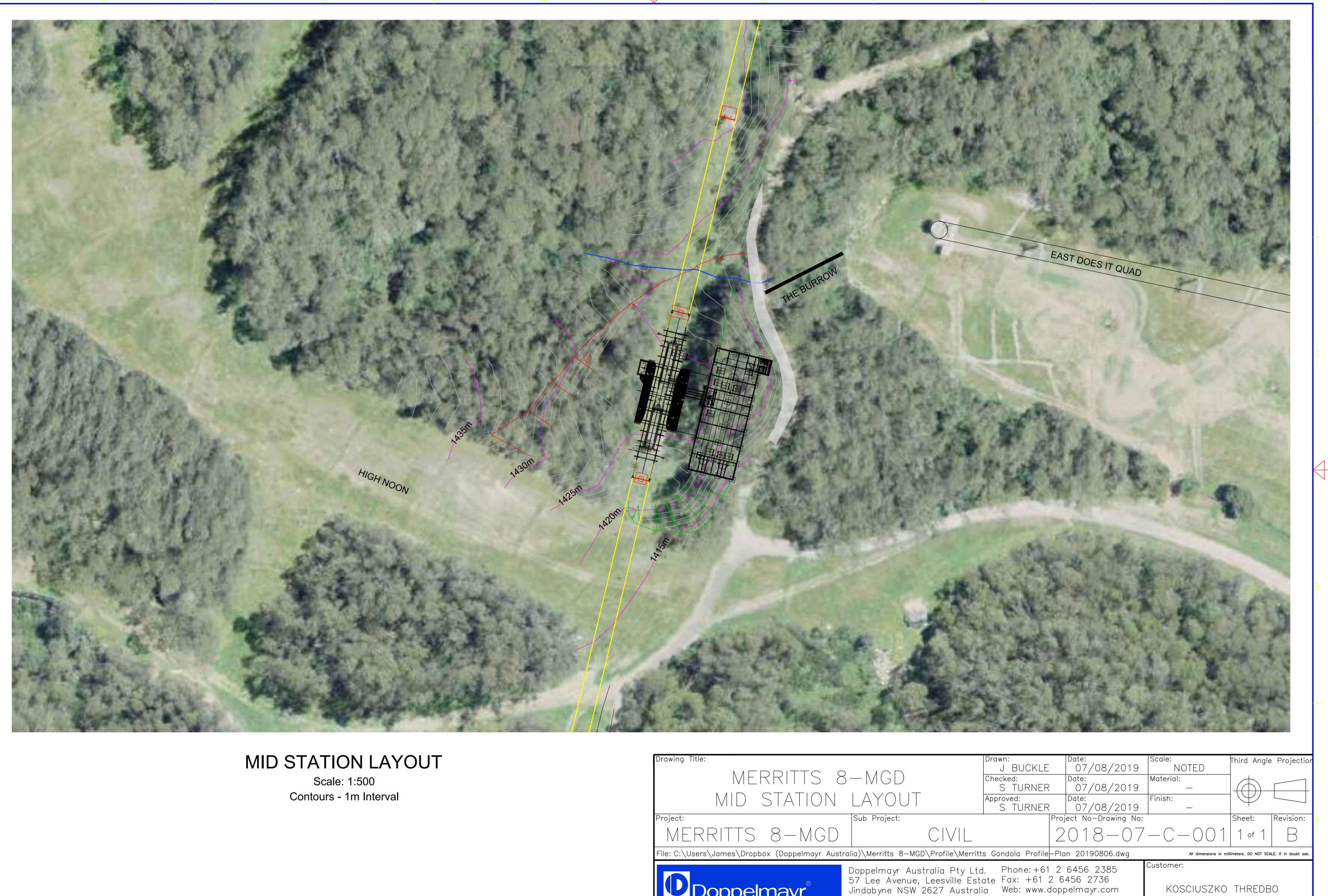


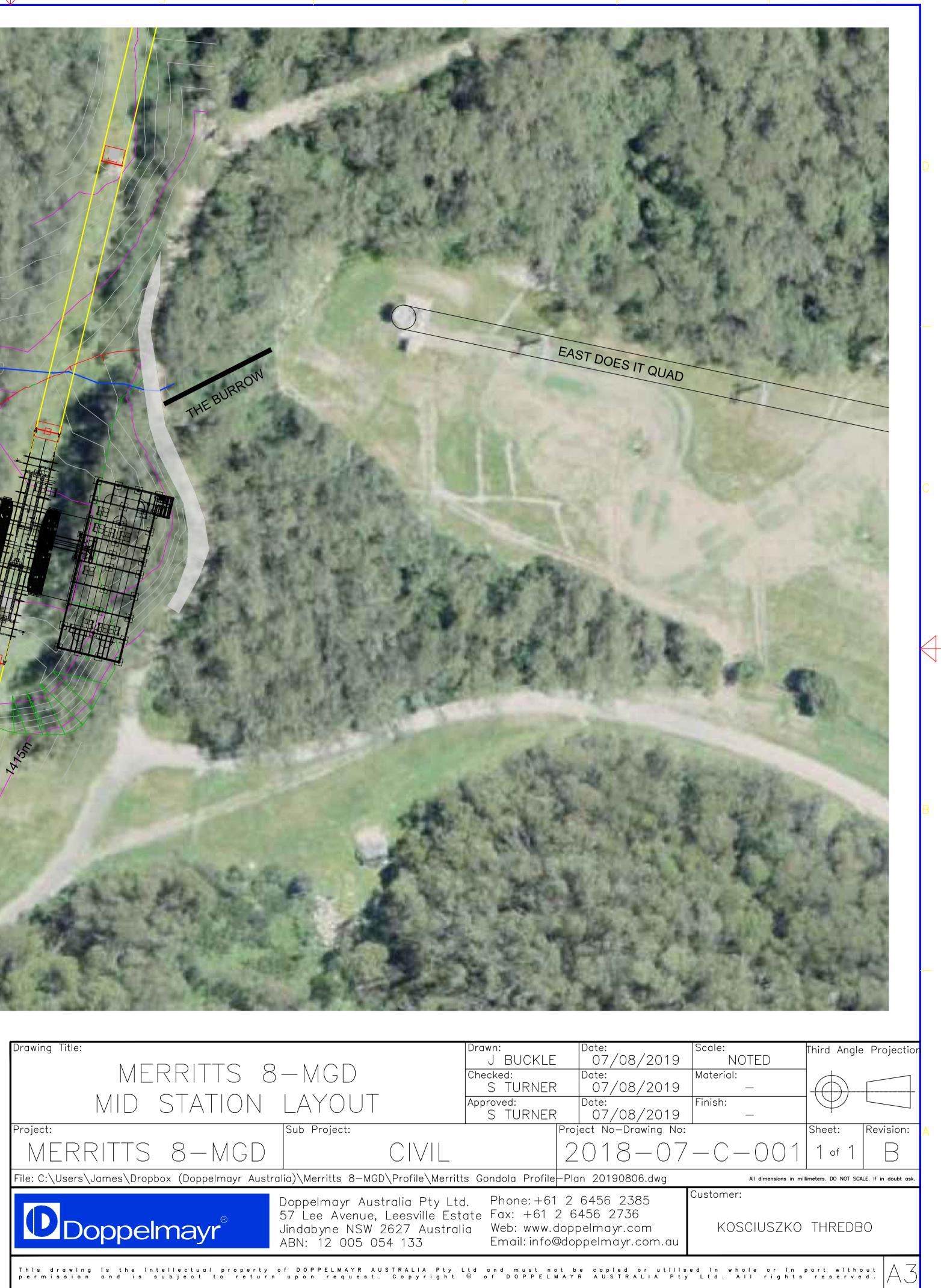


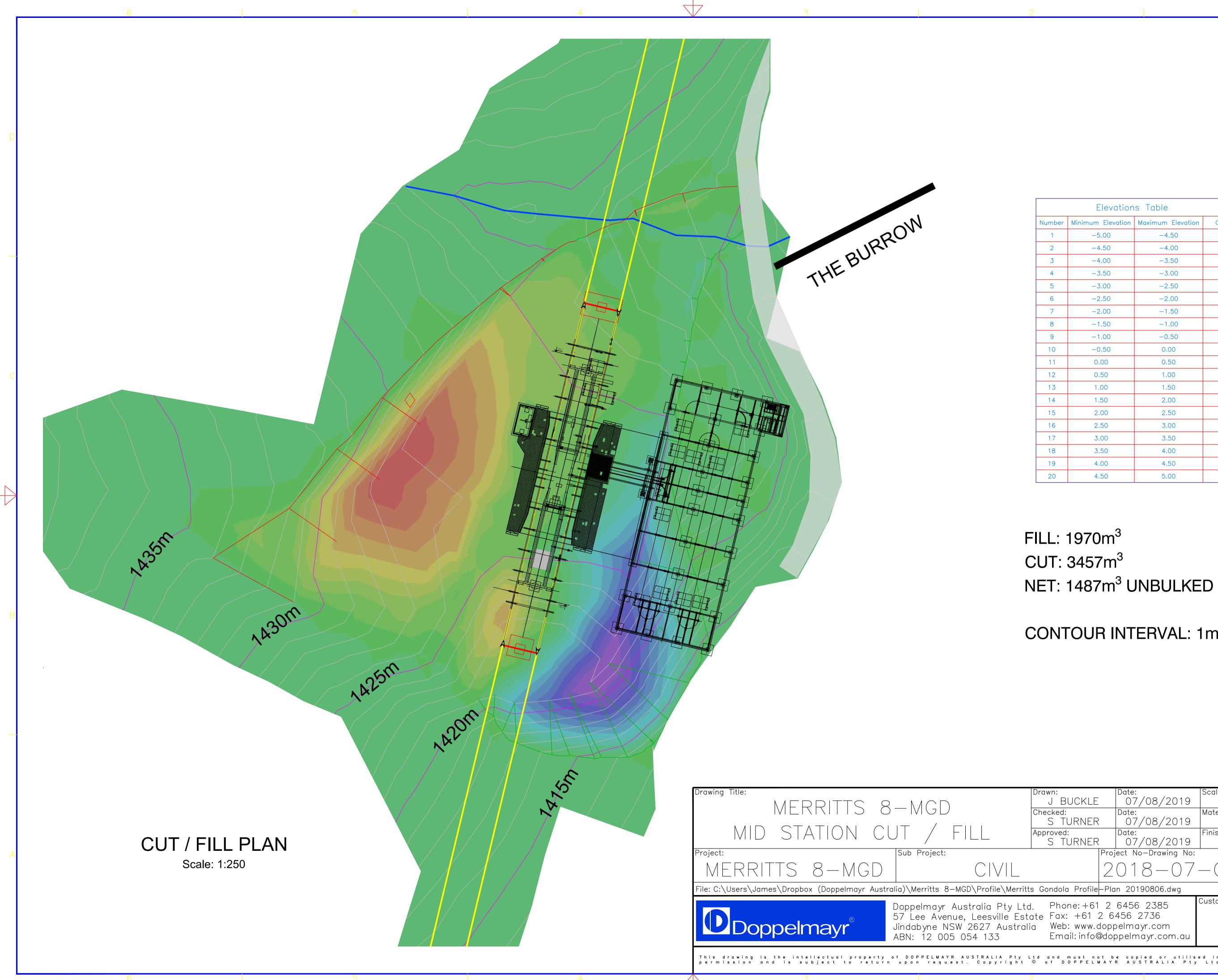












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# CONTOUR INTERVAL: 1m

	Elevation	s Table	
nber	Minimum Elevation	Maximum Elevation	Color
1	-5.00	-4.50	
2	-4.50	-4.00	
3	-4.00	-3.50	
ŀ	-3.50	-3.00	
5	-3.00	-2.50	
5	-2.50	-2.00	
7	-2.00	-1.50	
3	-1.50	-1.00	
)	-1.00	-0.50	
0	-0.50	0.00	
1	0.00	0.50	
2	0.50	1.00	
3	1.00	1.50	
4	1.50	2.00	
5	2.00	2.50	
6	2.50	3.00	
7	3.00	3.50	
8	3.50	4.00	
9	4.00	4.50	
0	4.50	5.00	

## FINISHED SURFACE PLAN

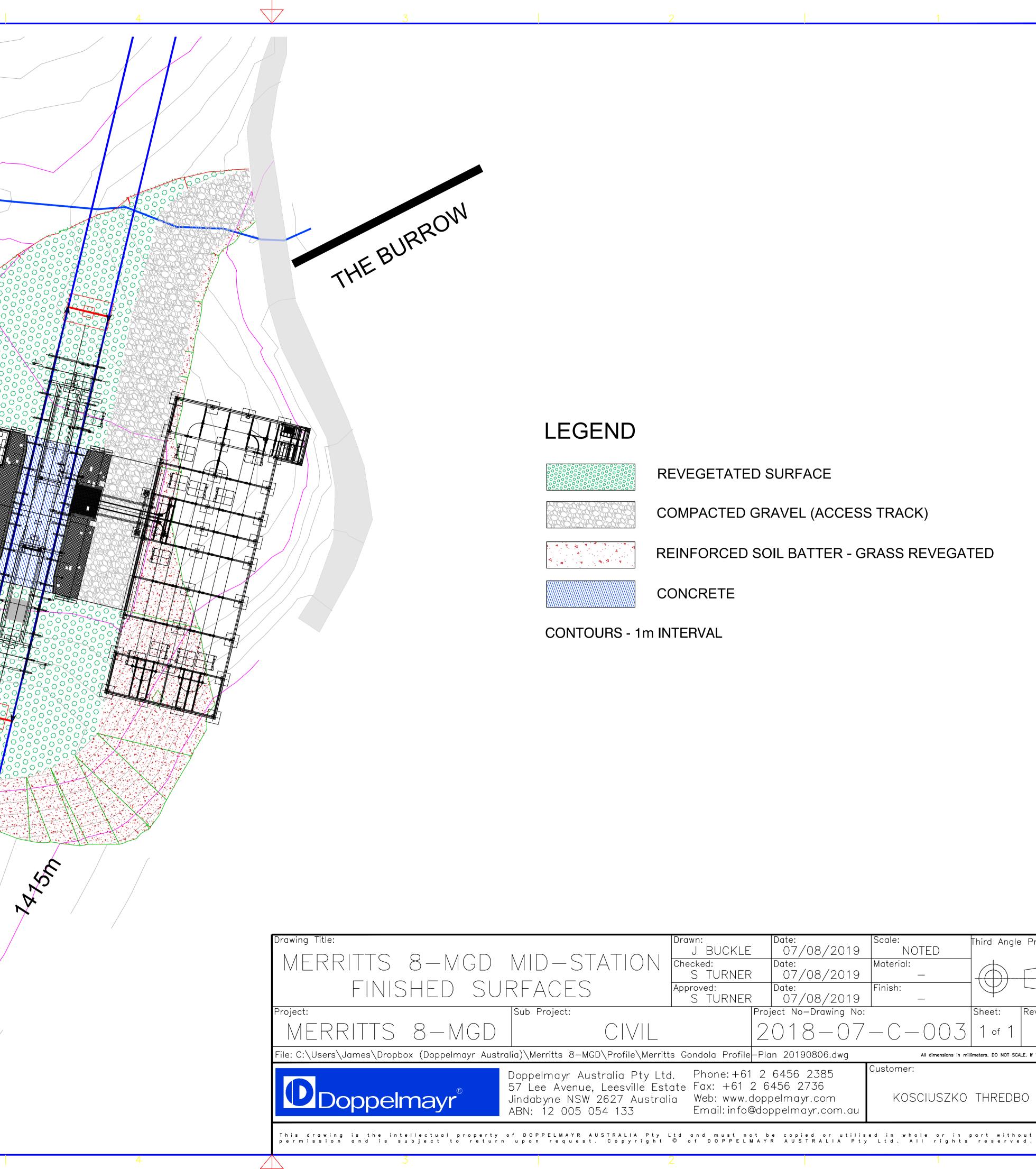
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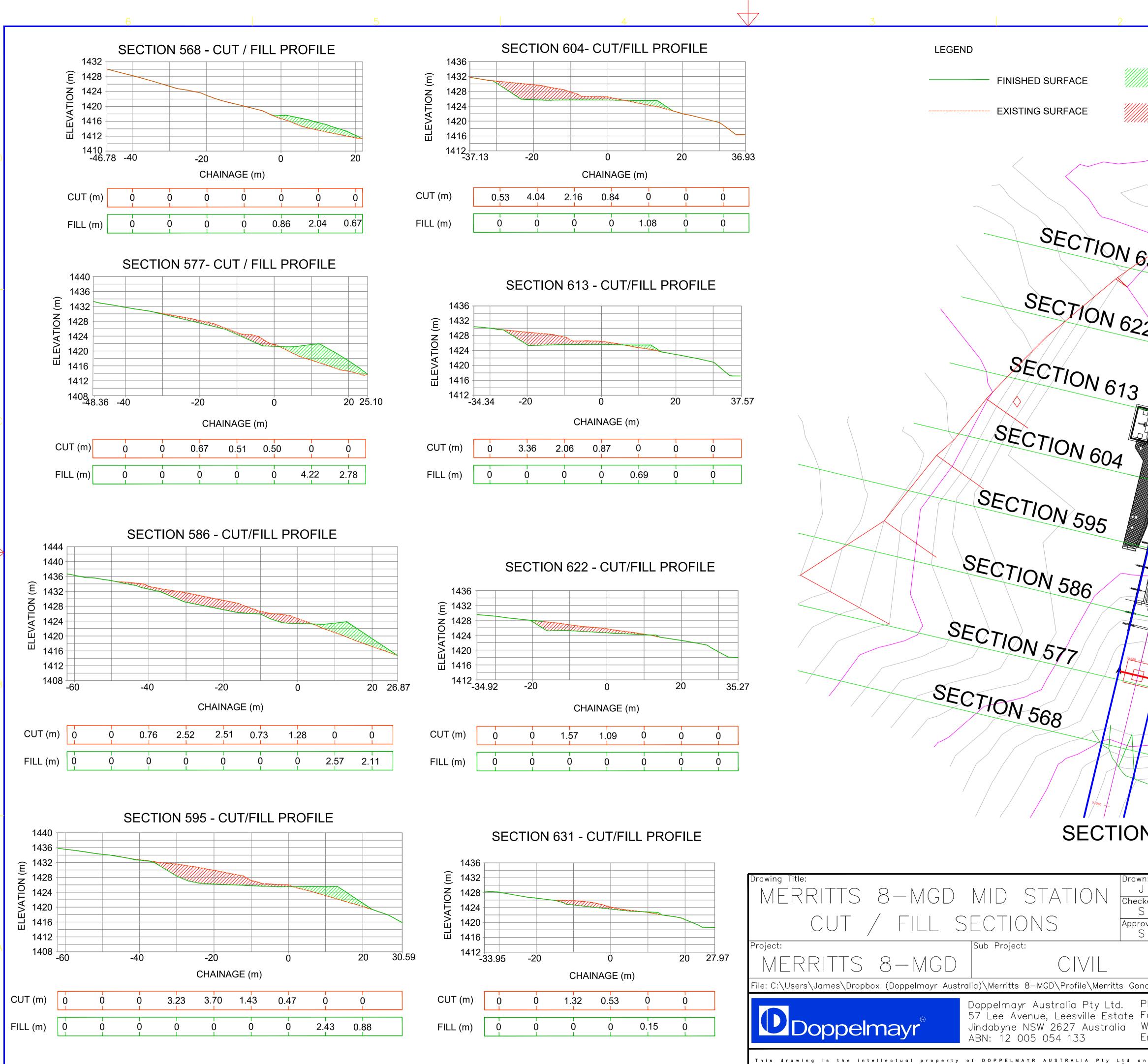
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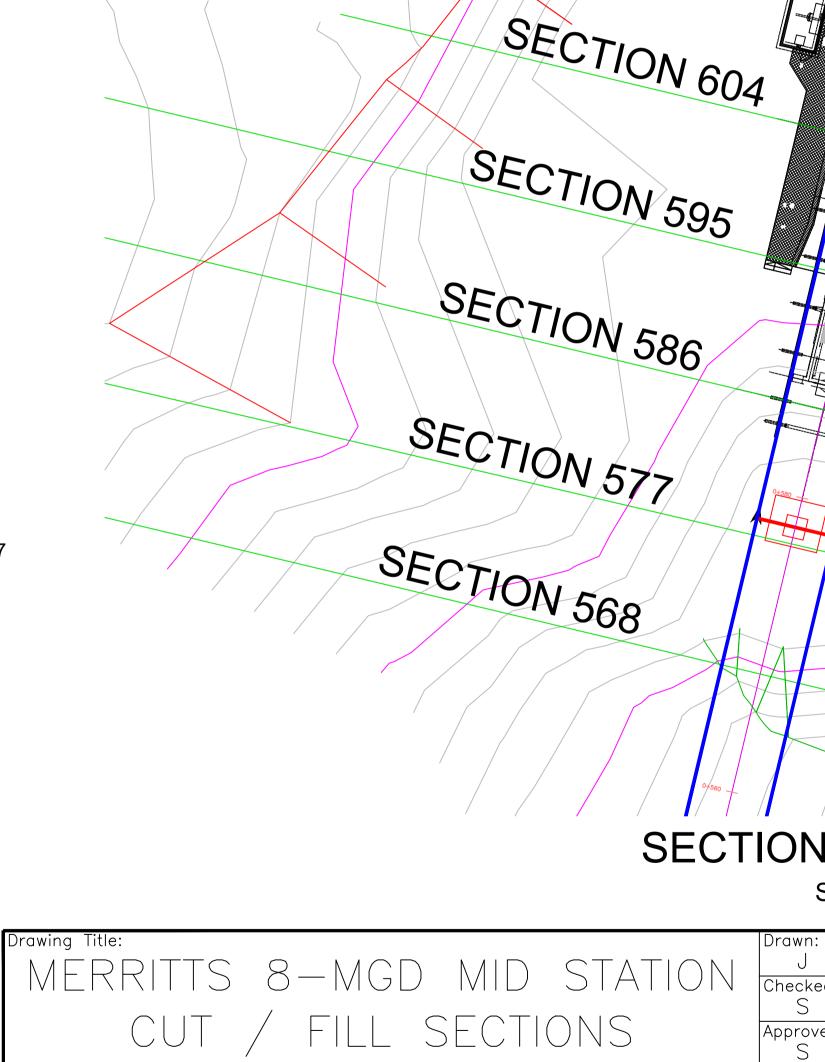
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Scale: 1:250



### Third Angle Projection Scale: Date: J BUCKLE 07/08/2019 NOTED Checked: S TURNER Date: Material: 07/08/2019 Date: Finish: S TURNER 07/08/2019 \_ Project No-Drawing No: Sheet: Revision: 2018 - 07 - C - 0Cof All dimensions in millimeters. DO NOT SCALE. If in doubt as Customer: KOSCIUSZKO THREDBO Email: info@doppelmayr.com.au





## LEGEND

Doppelmayr Jindabyne NSW 2627 Australia W ABN: 12 005 054 133 E

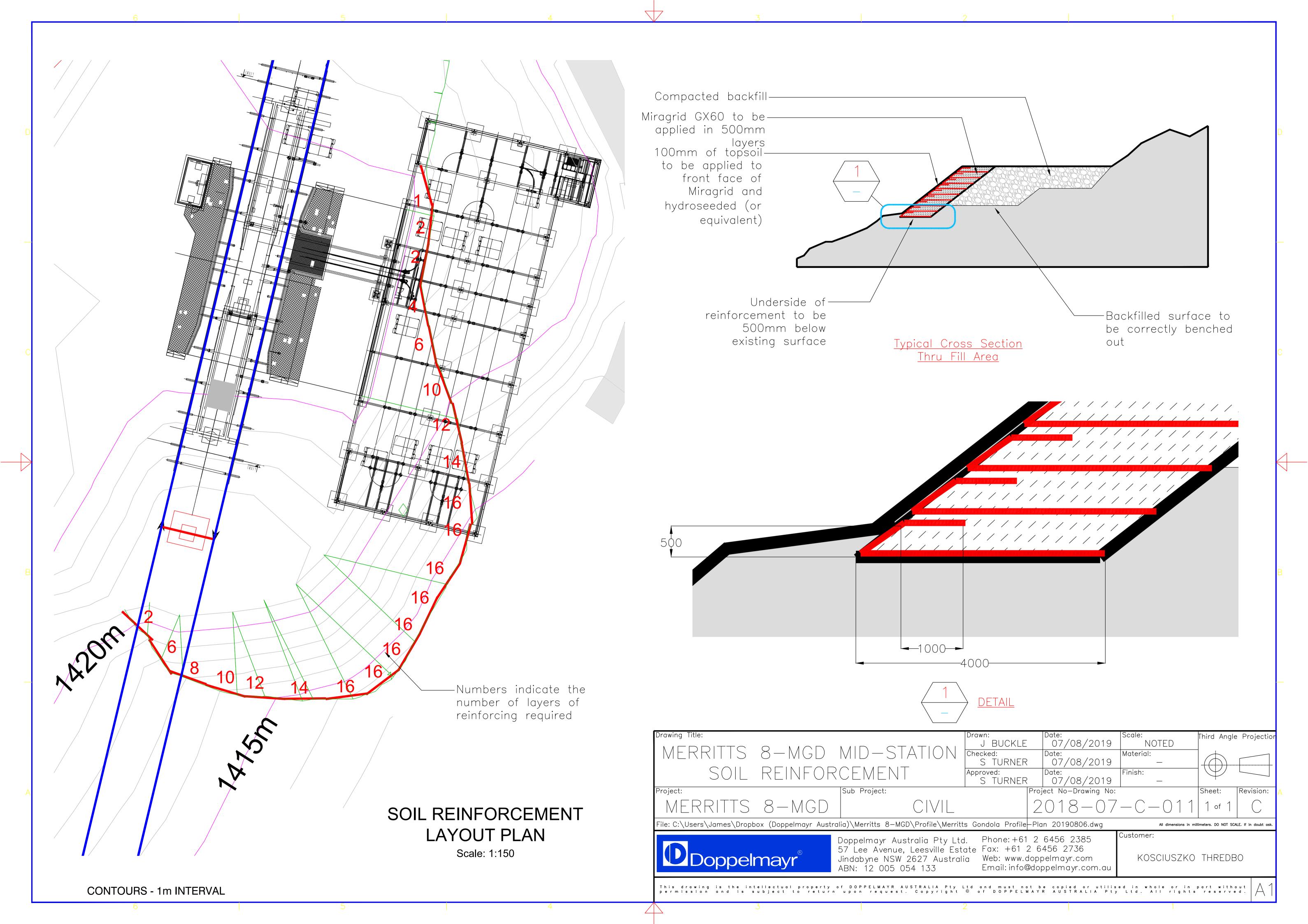
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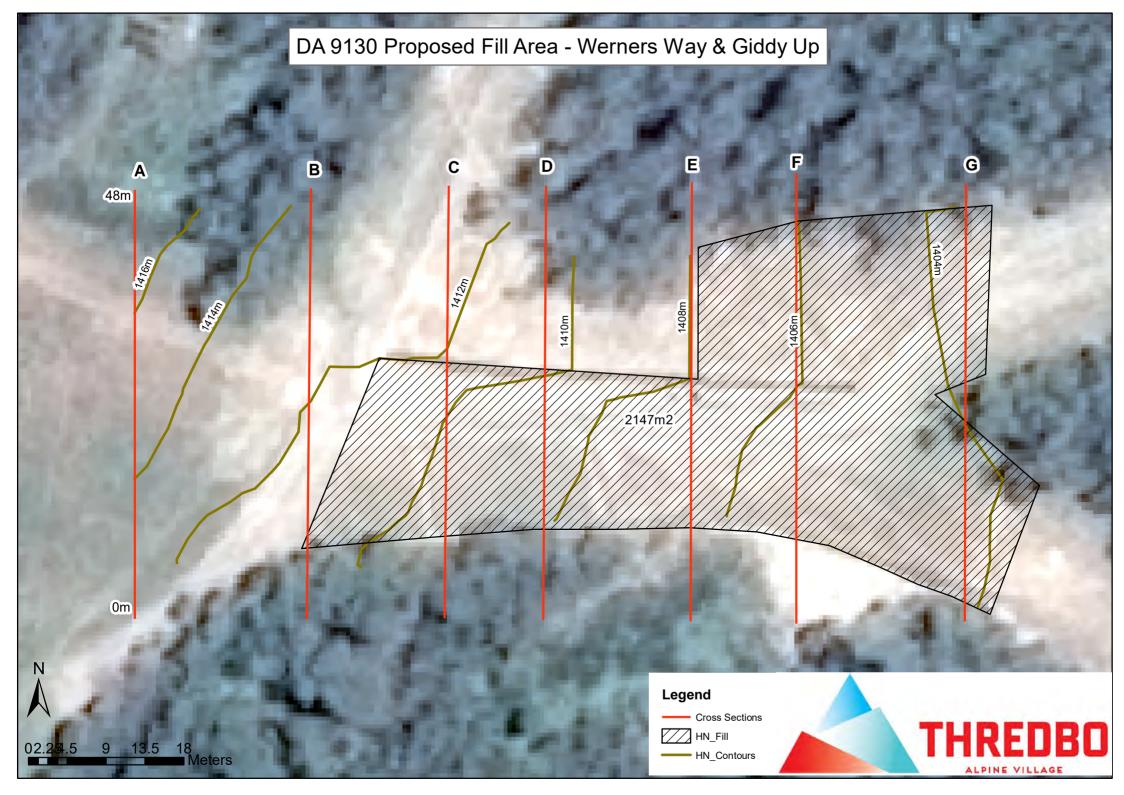
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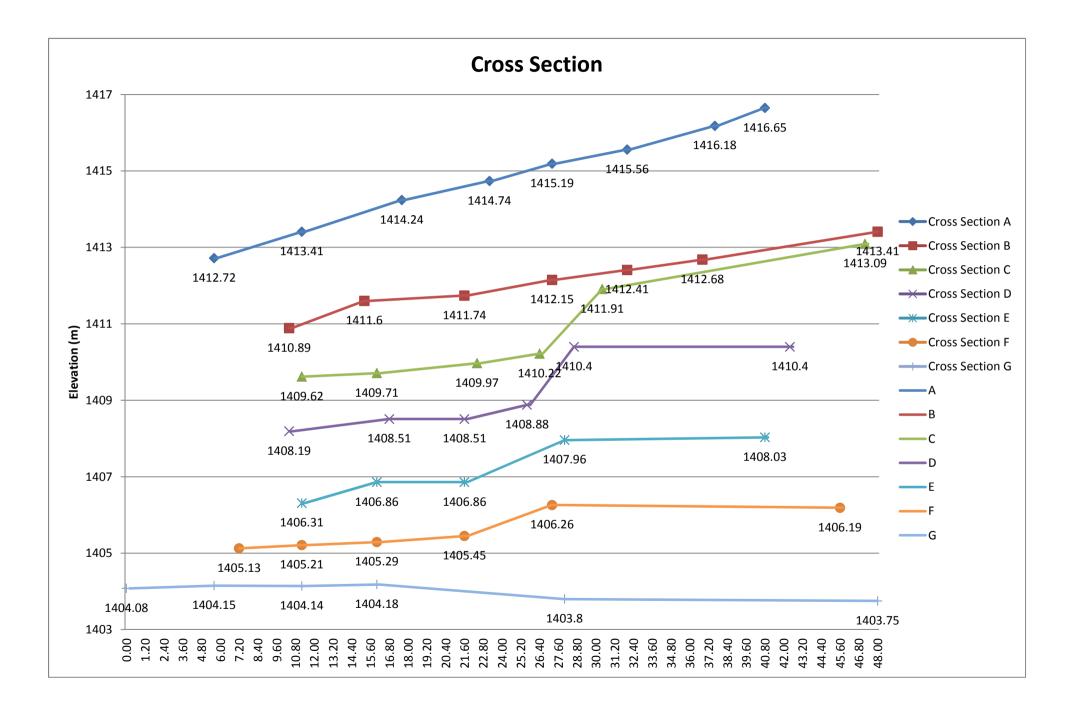
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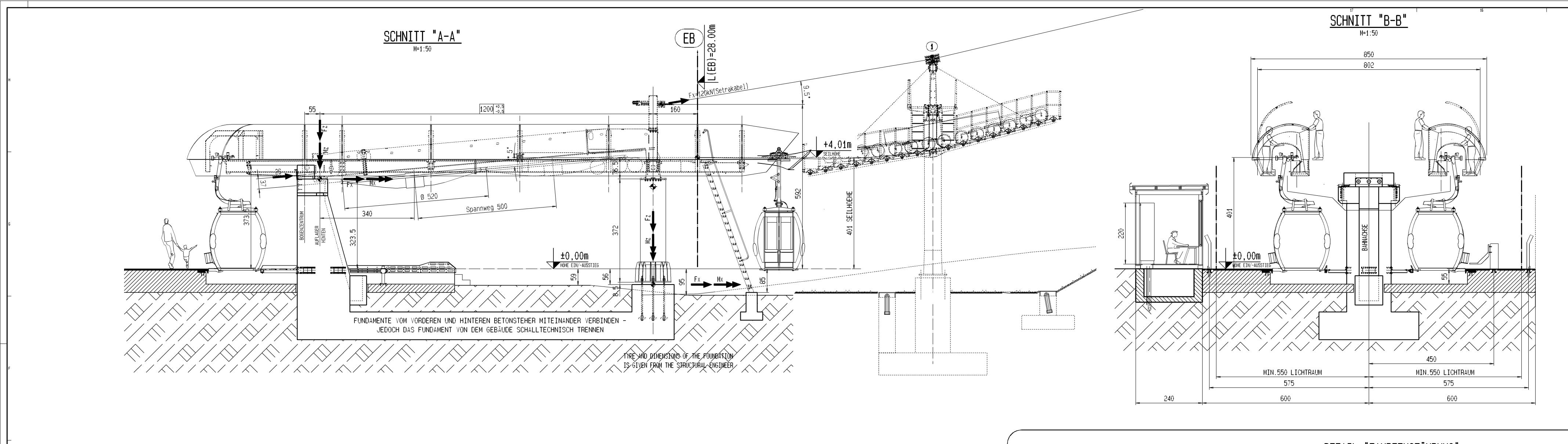
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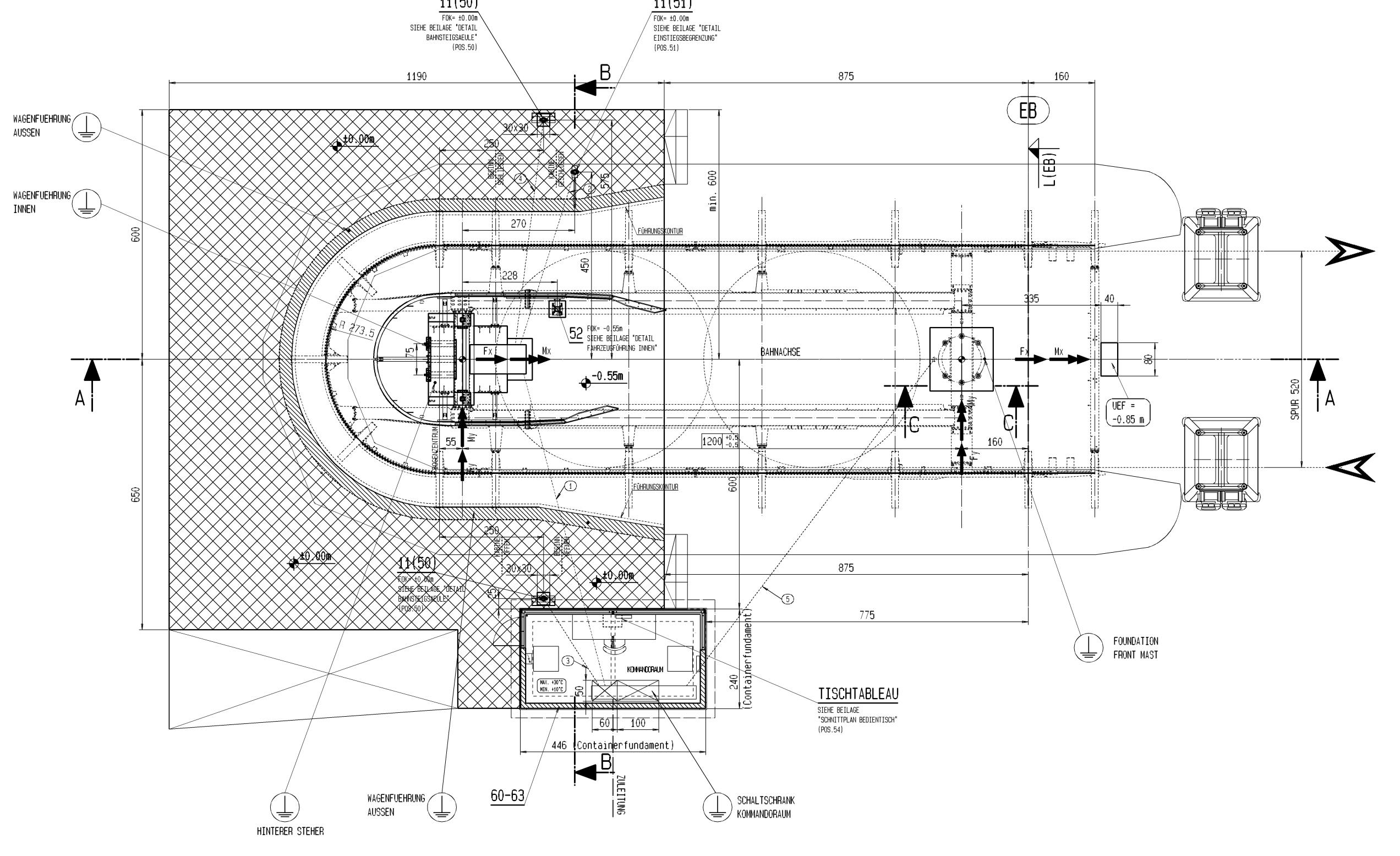
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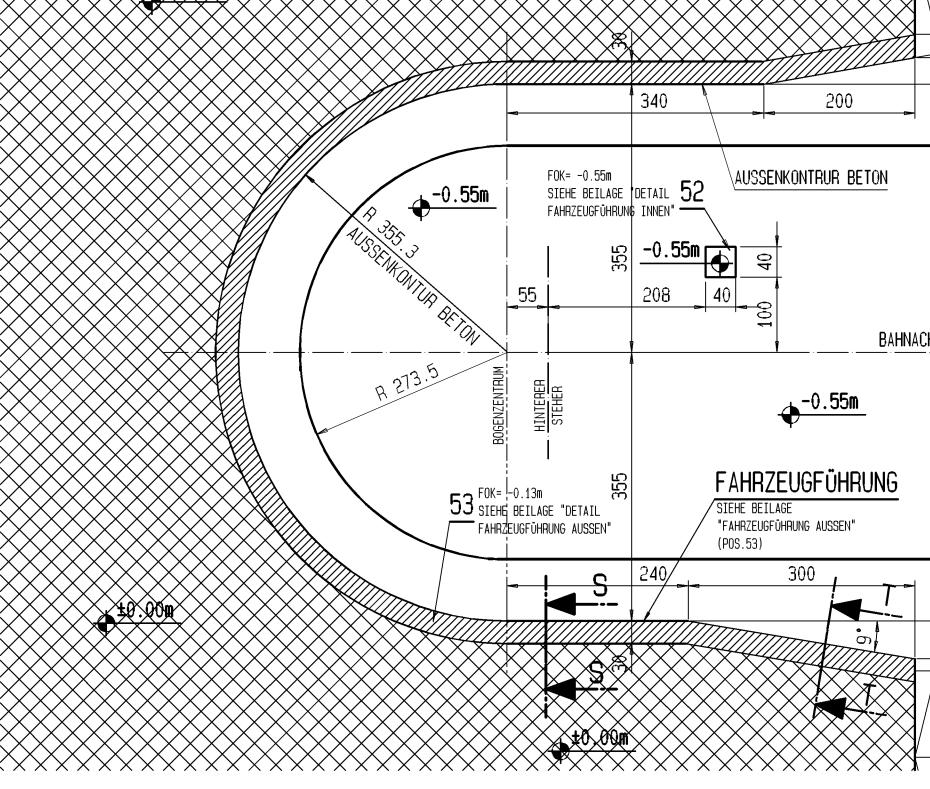


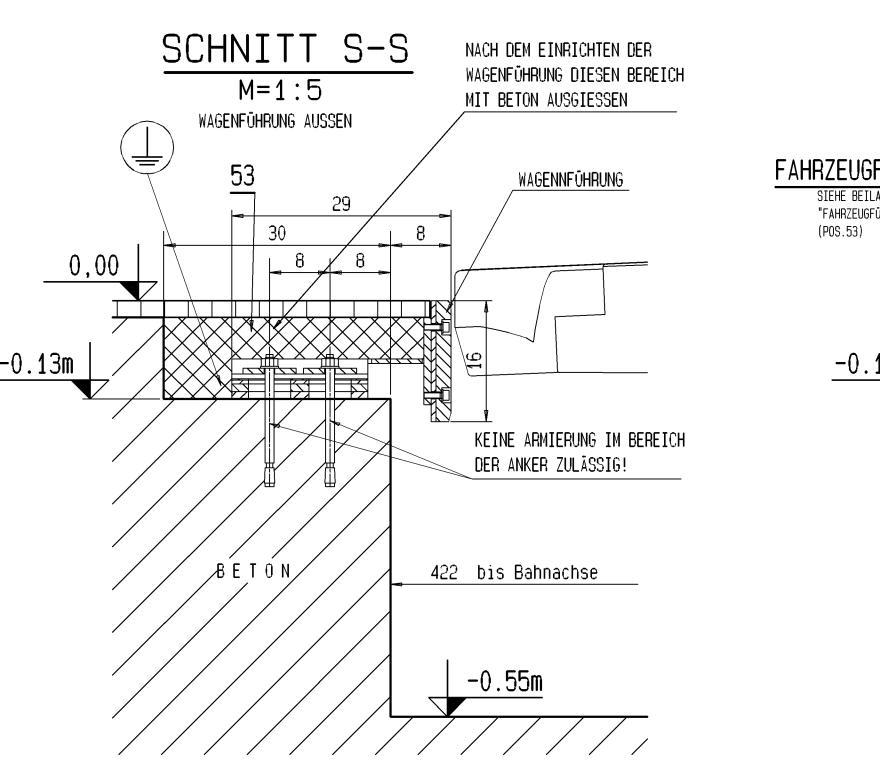


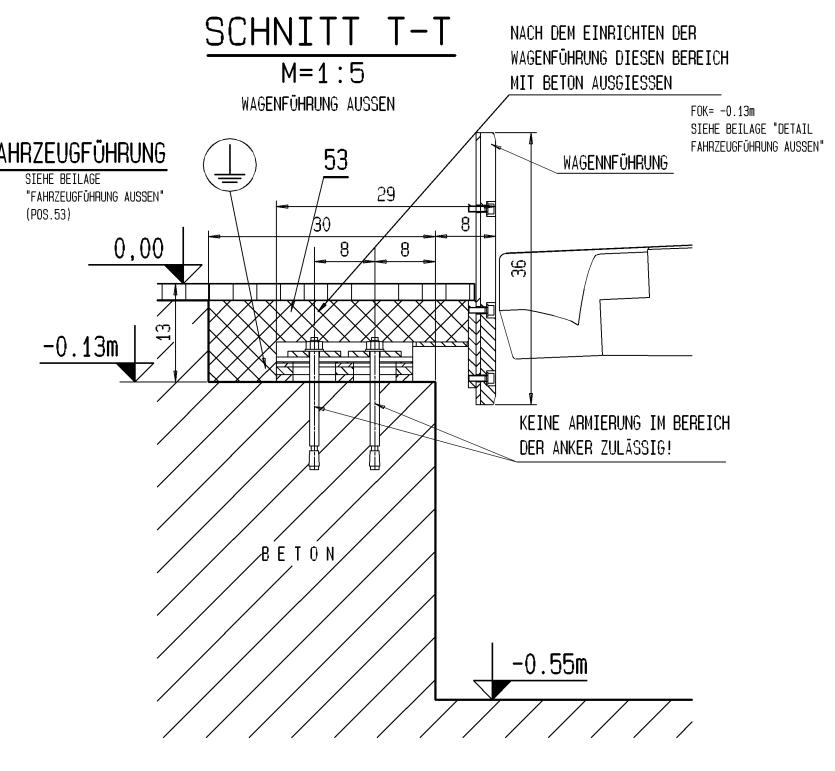


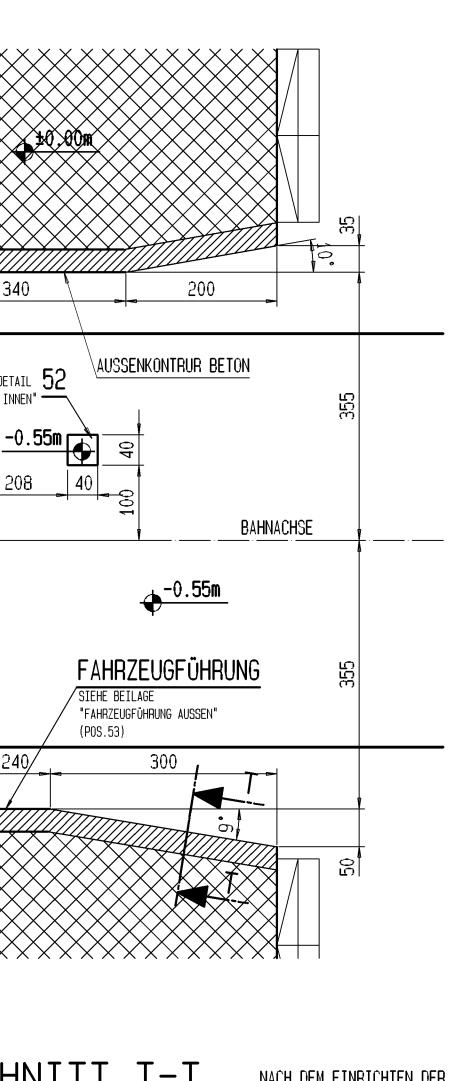


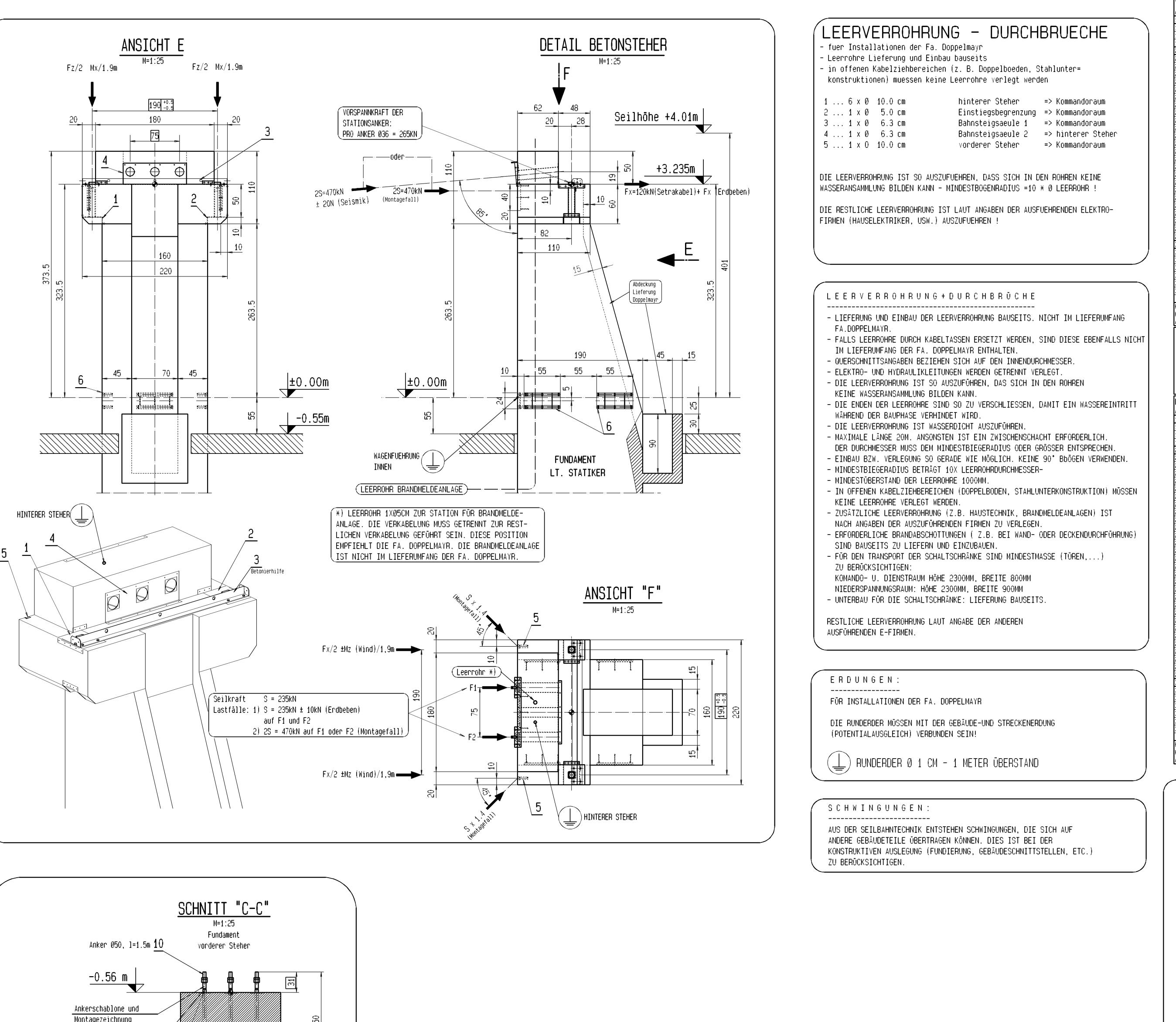
<u>DETAIL "FAHRZEUGFÜHRUNG"</u> 

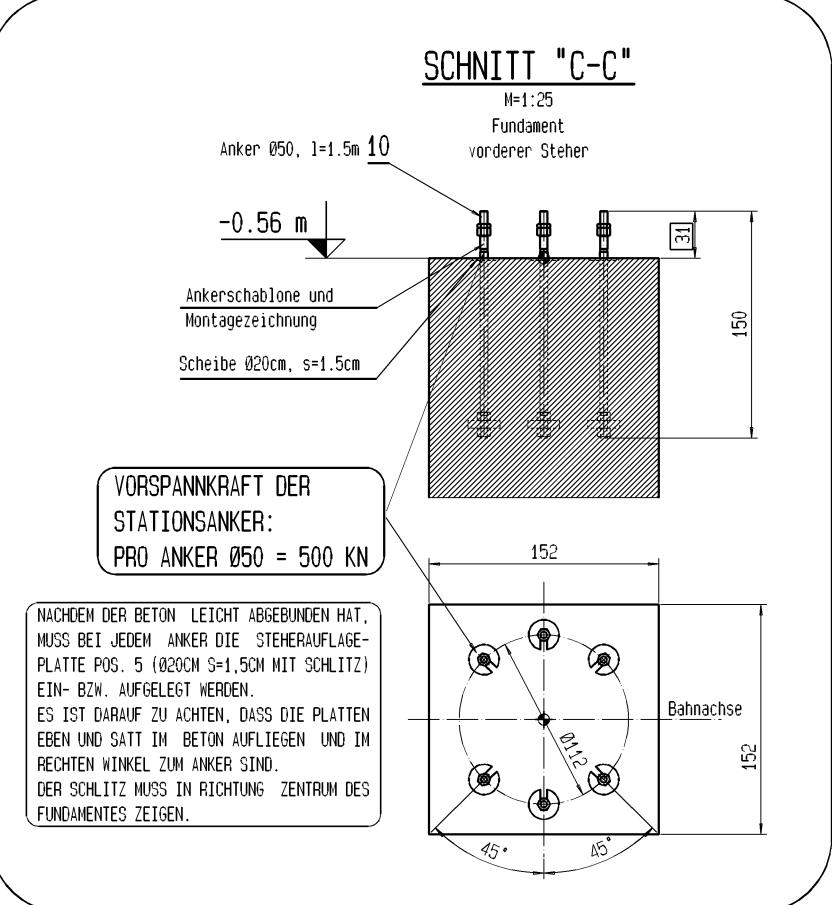




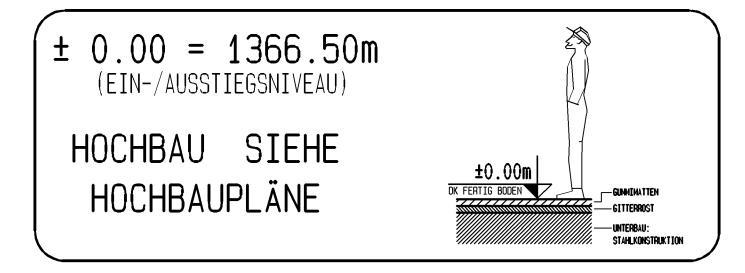








<u>15 14 13 8 12 7 11 6 10 5 </u>



POSITION	BENENNUNG	IDENTNUMMER	ZEICHNUNGSNUMMER
50	DETAIL BAHNSTEIGSAEULE	11459201	20168158E301101.F
51	DETAIL EINSTIEGSBEGRENZUNG	11481084	20174630E301101.B
52	DETAIL FAHRZEUGFUEHRUNG INNEN	11375685	20156453E301101.C
53	DETAIL FAHRZEUGFUEHRUNG AUSSEN	11497101	20179529E301101.B
54	SCHNITTPLAN BEDIENTISCH	11476032	20173305E003000.A
60	KOMANDORAUM	10760100	33001299D009902.B
61	SCHAL. UND BEWEHRUNGSPLAN FÜR CONTAINER	10713823	1010977 / 1011210 / 975445
62	ANSCHLUSSPLAN FÜR CONTAINER	10713817	33001289D009902.A
63	VERLEGEPLAN F. E+HYD. LEITUNGEN IM KABELKELLER	10787230	33001423D009904

Maßtoleranzen nach ÖNORM B 1100 / Toleranzklasse 16 / DIN 18202 Bl.4 (Maße in cm)
Dimensional tolerances as per ŌNORM B 1100 / tolerance class 16 / DIN 18202 page 4.
(dimensions in cm)

2	1					
Vorderer Steher	WAA00046	572		Tal	25/04	/2019
Anschlußkräfte am Anschlußpunkt (lt. Zeichnung)	8-MGD Me	errit	ts Gou	eloho		
1 Ständige Einwirkungen		Fy	Fz	MX	Μγ	Mz
1.1 Eigengewicht Seilbahntechnik	± 15	.,	180		± 50	
1.2 Eigengewicht Dach oder Deckel			40			
1.3						
2 Veränderliche Einwirkungen						
2.1 Eigengewicht durch fahrbaren Antrieb oder Umkehr						
2.1.1 Spannwagen vorne			85		± 10	
2.1.2 Spannwagen hinten			20		± 10	
2.2 Seile						
2.2.1 Seilablenkungen und Seilzug			45	± 0		
2.2.2 Telefon- oder Steuerseil			30			
2.3 Schnee (qs = 4.00 kN/m²)						
2.3.1 Beidseitig 100% Schnee			250		± 40	
2.3.2 Einseitig 100% Schnee			125	± 285	± 10	
2.3.3						
2.3.4						
2.4 Wind (qa = 1.00 kN/m²; qi = 0.25 kN/m²)						
2.4.1 Wind quer zur Bahnachse in Betrieb	1	± 10		± 40		
2.4.2 Wind quer zur Bahnachse außer Betrieb	<u>+</u>	± 35		± 160		
2.5 Wind auf 100 % Schnee						
2.5.1 Wind auf 100 % Schnee quer zur Bahnachse in Betrieb		± 5		± 30		
2.5.2 Wind auf 100 % Schnee quer zur Bahnachse außer Betrieb	1	± 20		± 115		
2.6						
2.6.1						
2.6.2						
2.7						
2.7.1						
2.7.2						
2.7.3						
3 Aussergewöhnliche Einwirkungen						
3.1 Seismische Einwirkungen (Erdbeben a_xy = 1.37 m/s²)						
3.1.1 Seismische Einwirkungen in x-Richtung			± 10			
3.1.2 Seismische Einwirkungen in y-Richtung		± 40		± 185		
3.1.3 Seismische Einwirkungen in z-Richtung			± 30			
3.2 Seilzug an einem Zylinder		± 15		± 75		
3.3 Überlast Spannhydraulik (Seilzugerhöhung)						
Kräfte und Momente gelten für den Endausbau						
Kräfte in [kN] und Momente in [kNm]						
Angaben der Kräfte und Momente ohne Teilsicherheitsbeiwerte						

x ... Bahnachse y ... Horizontalachse

z ... Vertikalachse

Hinterer Steher			WAAOOO	4672		Tal	25/0	4/2019
Anschlußkräfte am Anschlußpunkt (lt. Zeichnung)			8-MGD	Merri	ts Gor	ndola		
1 Ständige Einwirkungen			Fx	Fy	Fz	Мх	Му	Mz
1.1 Eigengewicht Seilbahntechnik					205	± 110		
1.2 Eigengewicht Dach oder Deckel					40			
1.3								
2 Veränderliche Einwirkungen								
2.1 Eigengewicht durch fahrbaren Antrieb oder Umkehr								
2.1.1 Spannwagen vorne					60			
2.1.2 Spannwagen hinten					125			
2.2 Seile								
2.2.1 Seilablenkungen und Seilzug	2 S	= 470 kN		siehe	Detail	Beton	steher	
2.2.2 Telefon- oder Steuerseil			120		-30			
2.3 Schnee (qs = 4.00 kN/m²)								
2.3.1 Beidseitig 100% Schnee					230			
2.3.2 Einseitig 100% Schnee					115	± 260		
2.3.3								
2.3.4								
2.4 Wind (qa = 1.00 kN/m²; qi = 0.25 kN/m²)								
2.4.1 Wind quer zur Bahnachse in Betrieb				± 10		± 10		± 20
2.4.2 Wind quer zur Bahnachse außer Betrieb				± 35		± 35		± 50
2.4.3 Wind in Bahnache in Betrieb			± 5					
2.4.4 Wind in Bahnache außer Betrieb			± 20					
2.5 Wind auf 100 % Schnee								
2.5.1 Wind auf 100 % Schnee quer zur Bahnachse in Betrieb				± 5		± 15		± 10
2.5.2 Wind auf 100 % Schnee quer zur Bahnachse außer Betrieb				± 20		± 45		± 30
2.5.3 Wind auf 100 % Schnee in Bahnache in Betrieb			± 5					ļ
2.5.4 Wind auf 100 % Schnee in Bahnache außer Betrieb			± 10					<b> </b>
2.6								<u> </u>
2.6.1								<u> </u>
2.6.2								<u> </u>
2.7								<b> </b>
2.7.1								<b> </b>
2.7.2								<b> </b>
2.7.3								<b> </b>
2.7.4								<u> </u>
3 Aussergewöhnliche Einwirkungen								
3.1 Seismische Einwirkungen (Erdbeben $a_xy = 1.37 \text{ m/s}^2$ )			1 70		1 10			<u> </u>
3.1.1 Seismische Einwirkungen in x-Richtung			± 70		± 10			<b> </b>
3.1.2 Seismische Einwirkungen in y-Richtung				± 50	1 JE	± 50		
3.1.3 Seismische Einwirkungen in z-Richtung				1 1 F	± 35	± 0		1 400
3.2 Seilzug an einem Zylinder	2.0	- 500 IM		± 15	L			<u>± 180</u>
3.3 Überlast Spannhydraulik (Seilzugerhöhung)	2 S	= 522 kN			Detail 	Berou	stener	
Kräfte und Momente gelten für den Endausbau				1	1			<u> </u>
Krāfte in [kN] und Momente in [kNm]								
Angaben der Kräfte und Momente ohne Teilsicherheitsbeiwerte							Rev. 19	(V 2.1)

### ALLGEMEIN: -----

\* KRÄFTE GELTEN FÜR ANFANGS UND ENDAUSBAU

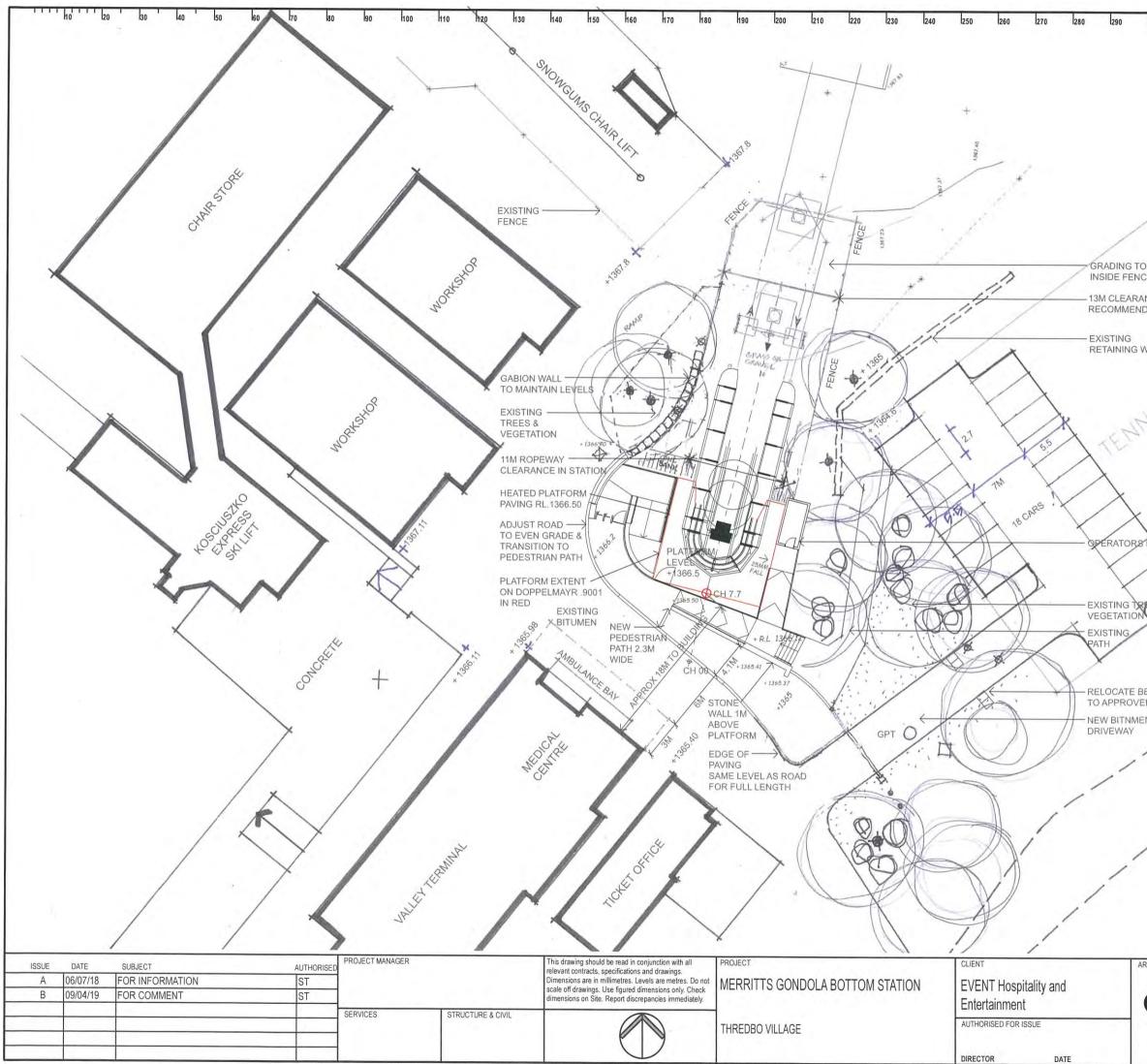
\* BEI ÖRTLICHEN KRAFTEINLEITUNGEN SIND ENTSPRECHENDE RÜCK= HALTE- UND SPALTZUGARMIERUNGEN ZU BERÜCKSICHTIGEN

\* WIND-, SCHNEELASTEN SIND ZU BEACHTEN!

- \* ZU DEN STATIONSAUFLAGERN, SCHALTSCHRÄNKEN, KABINENFÜHRUNGEN, HUBPODESTEN, GITTERROSTFLÄCHEN UND ABSTURZSICHERUNGEN SIND ENTSPRECHEND DEN VORSCHRIFTEN ERDUNGSBÄNDER ZU VERLEGEN (ÜBERSTAND MIND. 1 M)!
- \* AN ANKERSCHRAUBEN DARF NICHT GESCHWEISST ODER GEHEFTET WERDEN! ( BRUCHGEFAHR !!! )
- \* HYDRAULIK- UND ELEKTROLEITUNGEN WERDEN GETRENNT VERLEGT!
- \* ALLE MASSE IN CM
- \* LICHTE MINDESTMAßE (TÜREN, USW.) FÜR DEN ANTRANSPORT DER SCHALTSCHRÄNKE: H= 230 CM, B= 90 CM
- \* FÜR DIE HAUSELEKTRIK SIND ZUSÄTZLICHE LEERROHRE VORZUSEHEN !
- \* AUSFÜHRUNG HOCH- UND STAHLBAU LAUT PLÄNE FÜR HOCH-/STAHLBAU

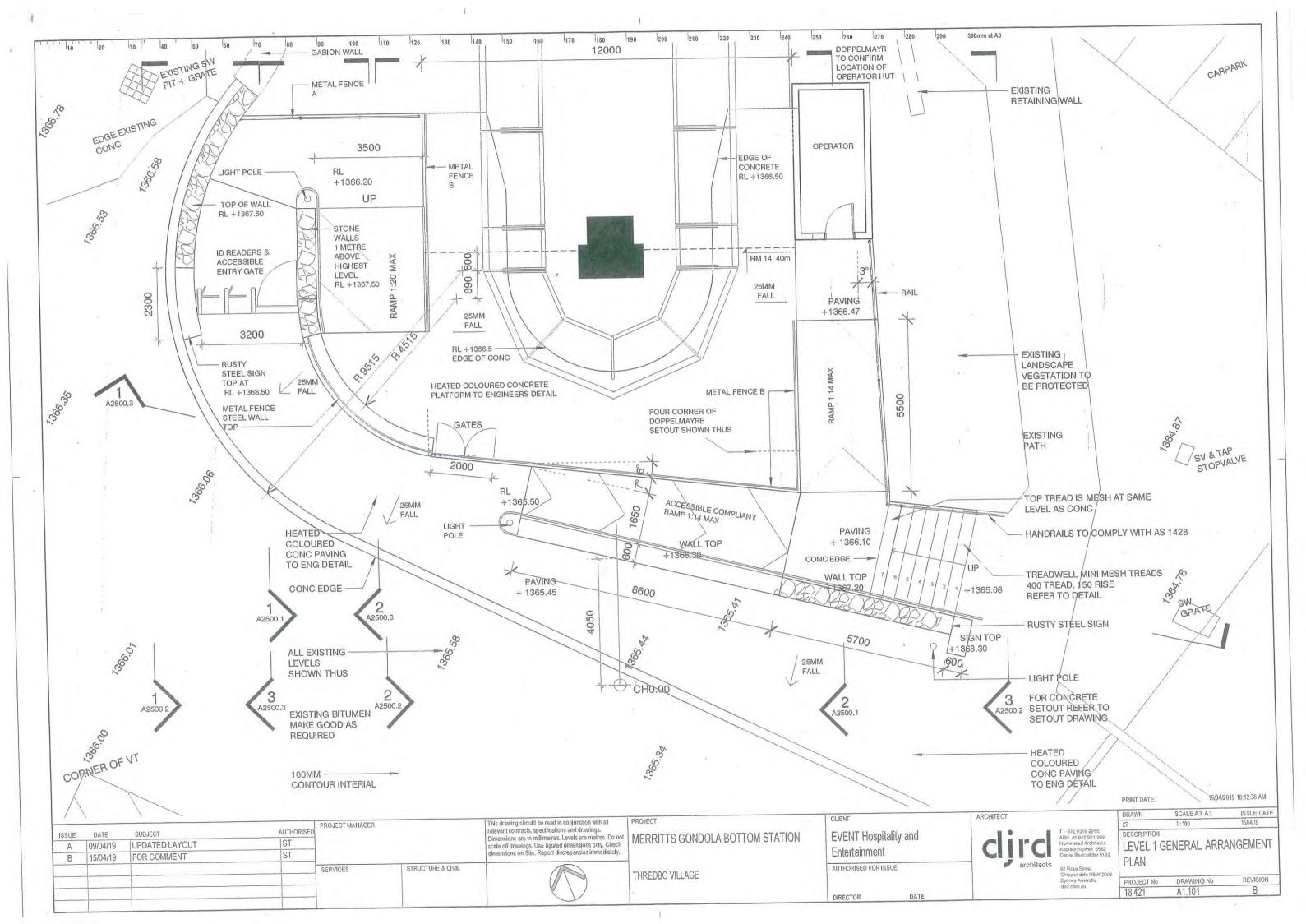
🕈 (Angriffspunkte der Kräfte lt Tabelle)

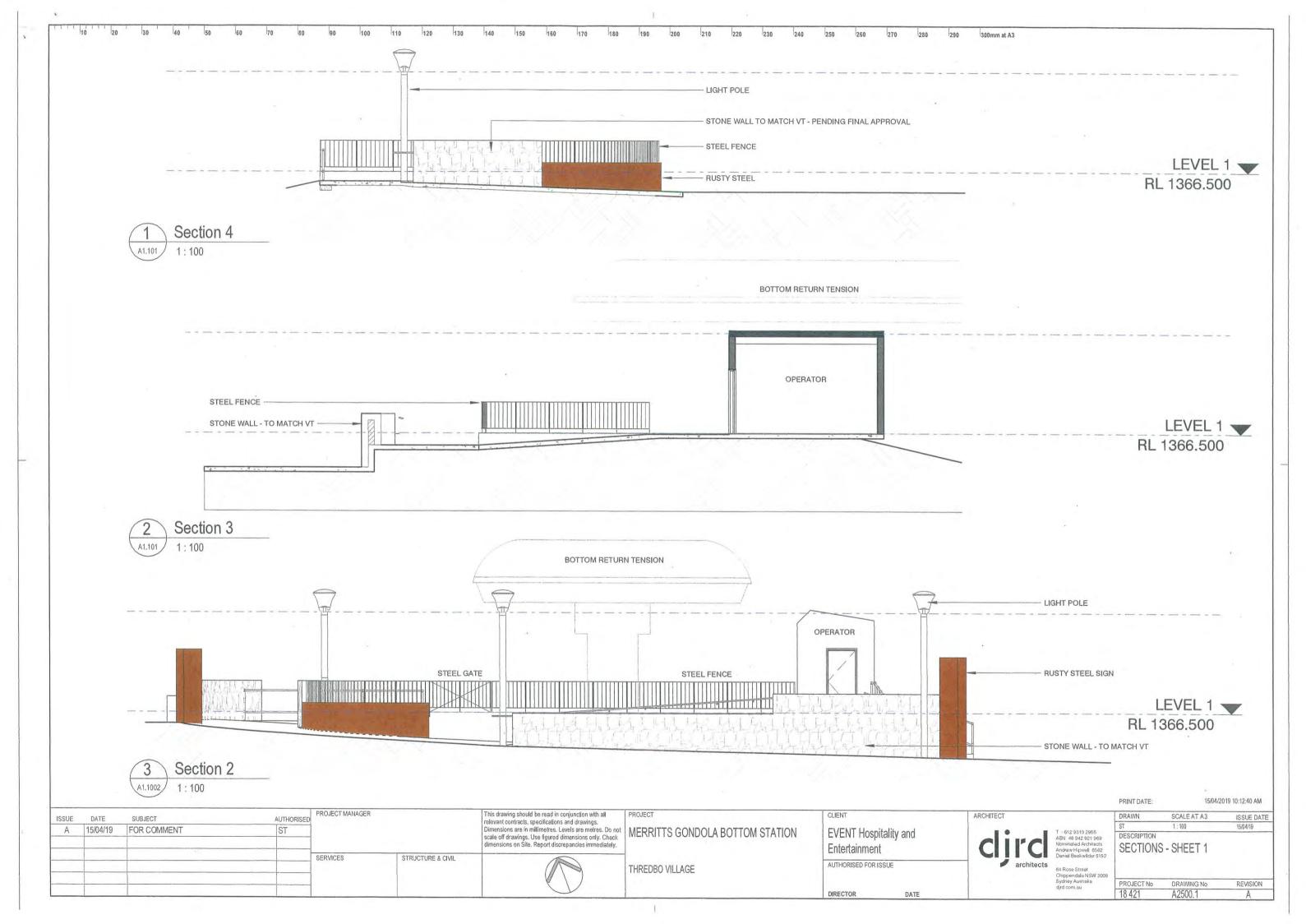
a	Staudruck für Windlast auf 1kN/m² erhöht!	2019-04-25	Meissner Armin				1	18537	764
Inde	Ānderung	Datum	Name	Ausführung				IDENT-NR	
	신 Doppelmayr Rickenba	Patent GmbH Icherstr. 8-10 IOLFURT/AUSTRIA	Q-Klasse:	$\bigcirc $	Maßstab 1/50 Faktor: 2.5	gezeichnel gepr. gesp.	Datum 2019-04-0 2019-05-09/13:	5 Meissr	ame her Arm her Armin
	lage <u>8-MGD MERRITTS</u> trag <u>WAA0004672</u>		Stück	Χ	<sup>Masse roh</sup> Kg		otted: root	5-10	RE
	ANSCHLUSSPLAN T	Ursprungszng. Nr. 20206510		01	IDENT-NR.	855704			
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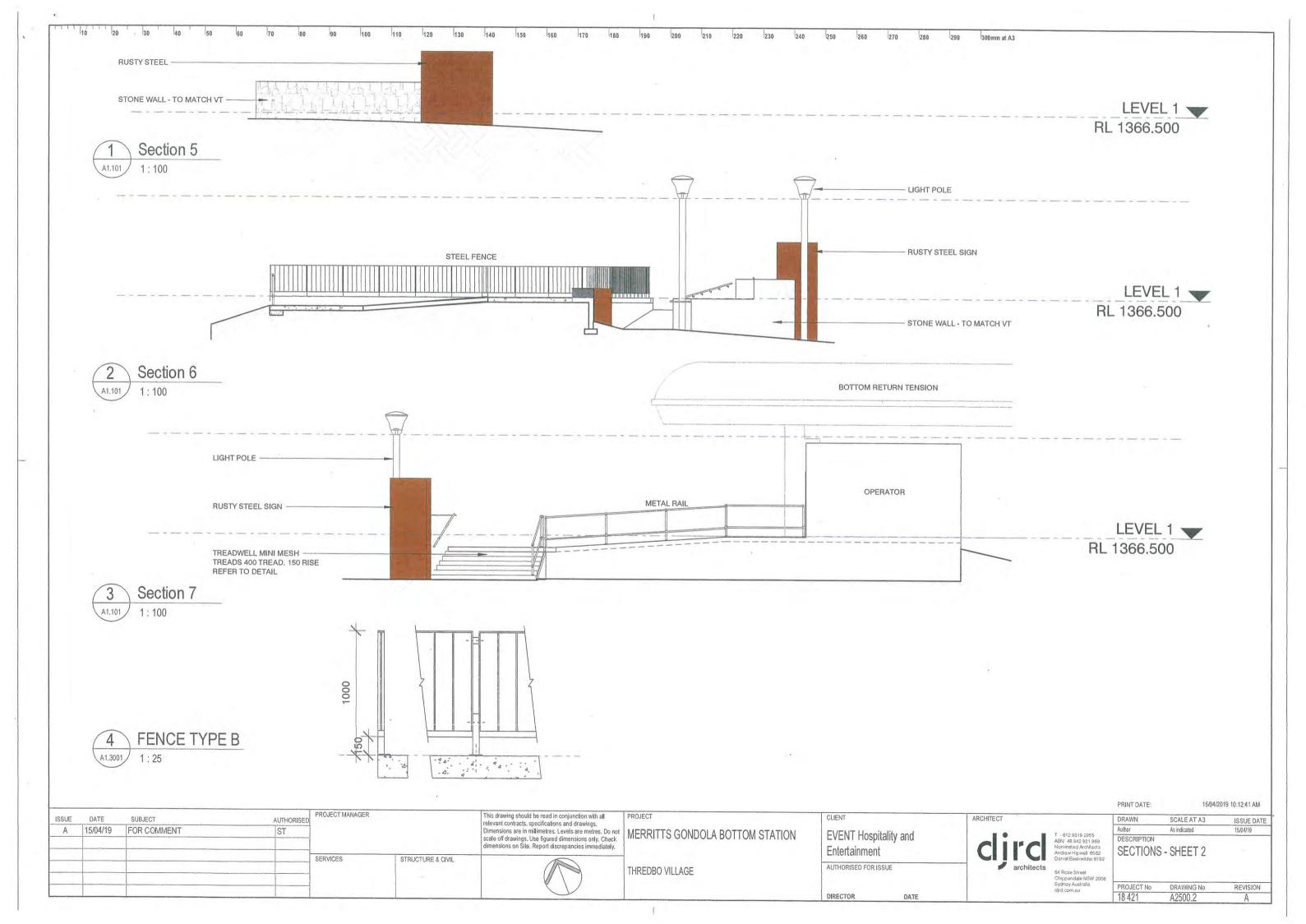


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64 Rose Street. Chippendale NSW 2008 Sydney Australia	000 150511	DB	
Syoney Australia djrd.com.au	PROJECT No 18 421	DRAWING No A1.100	REVISION

A1.100







1 10 20 30 40 50 100 110 120 130 140 60 150 160 170 180 190 200 220 70 80 90 210 230 240 250 260 270 280 290 BOTTOM RETURN TENSION \_\_\_\_ STEEL GATE STEEL FENCE Section 8 1 A1.101 1:100 METAL FENCE Section 1 2 1:100 A1.101 -6. . Section 9 3 A1.101 1:100 This drawing should be read in conjunction with all relevant contracts, specifications and drawings. Dimensions are in millimetres. Levels are metres. Do not scale off drawings. Use figured dimensions only. Check dimensions on Site. Report discrepancies immediately. PROJECT MANAGER PROJECT CLIENT ARC ISSUE DATE SUBJECT, AUTHORISED FOR COMMENT MERRITTS GONDOLA BOTTOM STATION A 15/04/19 ST EVENT Hospitality and Entertainment ( SERVICES STRUCTURE & CIVIL AUTHORISED FOR ISSUE THREDBO VILLAGE DIRECTOR DATE 1

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archite	64 Rose Street			
	Chippendale NSW 2008 Sydney Australia djrd.com.au	PROJECT No	DRAWING No	REVISION
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